



**GEOTEK ENGINEERING  
& TESTING SERVICES, INC.**

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Phone 605-335-5512 Fax 605-335-0773

January 3, 2017

Codington County  
14 1<sup>st</sup> Avenue SE  
Watertown, South Dakota 57201

Attn: Lee Gabel

Subj: Preliminary Geotechnical Exploration  
Proposed Justice Facility  
Codington County  
U.S. Highway 212  
Watertown, South Dakota  
GeoTek #16-E56

**Introduction**

This correspondence presents our reporting of the preliminary geotechnical exploration program for the referenced project. Our work was performed in accordance with your authorization.

**Project Location & Description**

The project site is located northwest of the intersection of U.S. Highway 212 and 25<sup>th</sup> Street SW in Watertown, South Dakota. Currently, the site is vacant and snow covered. A grade change of approximately 10 feet occurs across the site. The site slopes downward from the north to the south.

**Test Borings**

We performed six (6) test borings at the site on December 21, 2016. A test boring location map is attached showing the relative location of the test borings. The ground surface elevations at the test boring locations were determined by using the top of the fire hydrant located at the south edge of the site as a benchmark. An arbitrary elevation of 100.0 feet was used for the benchmark. A test boring location map, showing the relative location of the test borings, is attached at the conclusion of this report.

**Subsurface Conditions**

The subsurface conditions encountered at the test boring locations consisted of 1 ½ feet to 2 ½ feet of topsoil materials overlying fine alluvium soils and glacial till soils. The glacial till soils were the predominant soil type encountered and extended to the termination depth of the test borings.

The topsoil materials consisted of lean clay soils. The fine alluvium soils consisted of lean clay soils and lean clay with sand soils. The glacial till soils consisted of lean clay with sand soils and sandy lean clay soils.

The consistency or relative density of the soils is indicated by the standard penetration resistance (“N”) values as shown on the boring logs. A description of the soil consistency or relative density based on the “N” values can be found on the attached Soil Boring Symbols and Descriptive Terminology data sheet.

### **Groundwater Levels**

Measurements to record the groundwater levels were made at the test boring locations. The time and level of the groundwater readings are recorded on the boring logs. Groundwater was measured at a depth of 14 feet at test borings 2 and 3. Groundwater did not enter the boreholes at the remaining test borings at the time of our measurements.

The water levels indicated on the boring logs may or may not be an accurate indication of the depth or lack of subsurface groundwater. The limited length of observation restricts the accuracy of the measurements. A long period of time is generally required for subsurface water to stabilize in the boreholes. Long term groundwater monitoring was not included in our work scope.

### **Laboratory Testing**

Select samples from the test borings were submitted to the laboratory for testing. The tests consisted of moisture content, dry density and unconfined compressive strength. The results of the laboratory tests are shown on the boring logs adjacent to the samples upon which the tests were performed.

### **Project Information**

We understand that a new justice facility may be constructed at the site. The proposed justice facility will consist of a building and pavement areas. No loading information is available and no grading plans have been developed.

### **Discussion**

The test borings encountered 1 ½ feet to 2 ½ feet of topsoil materials overlying fine alluvium soils and glacial till soils. It is our opinion that the topsoil materials are not suitable for support of the footings or floor slab of the proposed building. Regarding the fine alluvium soils and glacial till soils, it is our opinion that these two (2) soil types have different supporting characteristics (net allowable soil bearing capacities). In our opinion, a net allowable soil bearing capacity of up to 2,000 pounds per square foot (psf) could be expected for the fine alluvium soils, while a net allowable soil bearing capacity of up to 3,500 psf could be expected for the glacial till soils.

In our opinion, the site preparation within the footing areas of the building would depend on which net allowable soil bearing pressure is used for the design of the footings. If a net allowable soil bearing pressure of up to 2,000 psf is used, then the site preparation in the footing areas of the building would likely consist of removing the topsoil materials in order to expose the fine alluvium soils or glacial till soils. If a net allowable soil bearing pressure of up to 3,500 psf is used, then the

site preparation in the footing areas of the building would likely consist of removing the topsoil materials and fine alluvium soils in order to expose the glacial till soils. If the excavation required to expose the fine alluvium soils or glacial till soils extends below the bottom-of-footing elevation, then granular structural fill would need to be placed and compacted up to the bottom-of-footing elevation. If soft/wet soils are encountered at the bottom of the excavations, then drainage rock may be needed at the bottom of the excavated areas.

With the fine alluvium soils (net allowable soil bearing pressure of up to 2,000 psf), we recommend limiting column loads to 100 kips and wall/strip loads of 4 kips per lineal foot. With the glacial till soils (net allowable soil bearing pressure of up to 3,500 psf), we recommend limiting column loads to 200 kips and wall/strip loads of 8 kips per lineal foot.

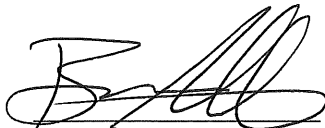
In the floor slab areas of the building, the site preparation would likely consist of removing the topsoil materials in order to expose the fine alluvium soils or glacial till soils. If the excavation required to expose the fine alluvium soils or glacial till soils extends below the bottom-of-slab elevation, then granular structural fill would need to be placed and compacted up to the bottom-of-slab elevation.

As an option, the on-site clay materials could be used as fill during initial site grading to achieve the design elevation within the building footprint. This option would depend on the grading plan, the floor slab loads and the net allowable soil bearing pressure used. Once specific plans are developed, we could provide comments and recommendations regarding the use of the on-site clay materials as fill.

### Remarks

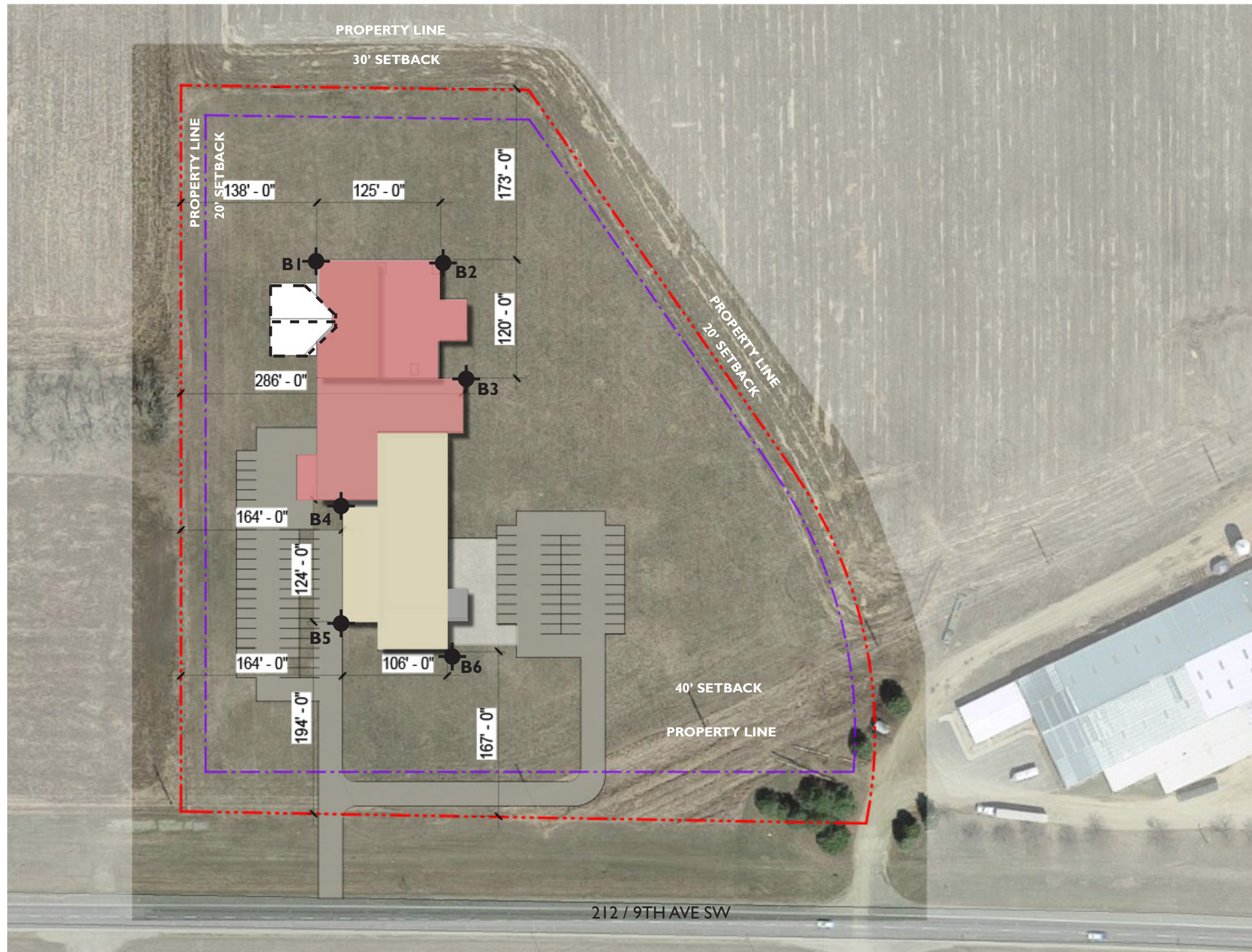
As specific plans are developed, we recommend that additional test borings and laboratory tests be performed for each lot. We trust this report provides you with the necessary information for the project. If you have any questions or require additional information, please contact our office.

GeoTek Engineering & Testing Services, Inc.

  
Brennen Ahlers, PE  
Project Manager







SCALE: 1" = 100'-0"

# Codington County Justice Facilities Study

SOIL BORING LOCATIONS

September 20, 2016 Project #: 2117.01



Codington County  
South Dakota

BKV  
GROUP



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**GEOTECHNICAL TEST BORING LOG**

GEOTEK # 16-E56

BORING NO. 1 (1 of 1)

PROJECT Preliminary Geotechnical Exploration, Proposed Codington County Justice Facility, US Highway 212, Watertown, SD

DEPTH in FEET	DESCRIPTION OF MATERIAL	GEOLOGIC ORIGIN	N	WL	SAMPLE		LABORATORY TESTS								
					NO.	TYPE	WC	D	LL	PL	QU				
	↓ SURFACE ELEVATION <u>107.1 ft</u>														
2½	<b>LEAN CLAY:</b> very dark brown, frozen to 1' then moist, (CL)	TOPSOIL			1	HSA									
3½	<b>LEAN CLAY:</b> brown, moist, firm, (CL)	FINE ALLUVIUM	6		2	SPT	24	100							
6	<b>LEAN CLAY:</b> mottled brown and gray, moist, firm, (CL)	FINE ALLUVIUM	8		3	SPT	26	95							
	<b>LEAN CLAY WITH SAND:</b> a little gravel, mottled brown and gray, moist, stiff, (CL)	GLACIAL TILL	9		4	SPT									
			9		5	SPT									
			9		6	SPT									
14½	<b>LEAN CLAY:</b> brown, moist, stiff, (CL)	GLACIAL TILL	10		7	SPT									
19½	<b>LEAN CLAY WITH SAND:</b> brown, moist, stiff, (CL)	GLACIAL TILL	14		8	SPT									
21	Bottom of borehole at 21 feet.														

GEOTECHNICAL TEST BORING 16-E56.GPJ GEOTEKENG.GDT 12/30/16

WATER LEVEL MEASUREMENTS

START 12-21-16 COMPLETE 12-21-16 12:44 pm

DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	WATER LEVEL	METHOD
12-21-16	2:34 pm	21	--	19	none	3.25" ID Hollow Stem Auger
--	--	--	--	--	--	
--	--	--	--	--	--	
--	--	--	--	--	--	CREW CHIEF Mike Wagner







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**GEOTECHNICAL TEST BORING LOG**

GEOTEK # <u>16-E56</u>						BORING NO. <u>3 (1 of 1)</u>											
PROJECT <u>Preliminary Geotechnical Exploration, Proposed Codington County Justice Facility, US Highway 212, Watertown, SD</u>																	
DEPTH in FEET	DESCRIPTION OF MATERIAL SURFACE ELEVATION <u>105.9 ft</u>					GEOLOGIC ORIGIN	N	WL	SAMPLE		LABORATORY TESTS						
									NO.	TYPE	WC	D	LL	PL	QU		
	<b>LEAN CLAY:</b> very dark brown, frozen to 1' then moist, (CL)					TOPSOIL			1	HSA							
2½	<b>LEAN CLAY:</b> brown, moist, firm, (CL)					FINE ALLUVIUM	5		2	SPT							
4½	<b>SANDY LEAN CLAY:</b> a little gravel, mottled brown and gray, moist, firm, (CL)					GLACIAL TILL	8		3	SPT							
							7		4	SPT							
							6		5	SPT							
							7		6	SPT							
14½	<b>LEAN CLAY WITH SAND:</b> a little gravel, mottled brown and gray, moist, firm to stiff, (CL)					GLACIAL TILL	8		7	SPT							
21	Bottom of borehole at 21 feet.						11		8	SPT							
WATER LEVEL MEASUREMENTS							START	<u>12-21-16</u>	COMPLETE	<u>12-21-16 11:10 am</u>							
DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	WATER LEVEL	METHOD											
12-21-16	2:39 pm	21	--	18	14	3.25" ID Hollow Stem Auger											
--	--	--	--	--	--												
--	--	--	--	--	--												
--	--	--	--	--	--	CREW CHIEF	Mike Wagner										

GEOTECHNICAL TEST BORING 16-E56.GPJ GEOTEKENG.GDT 12/30/16



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**GEOTECHNICAL TEST BORING LOG**

GEOTEK # 16-E56

BORING NO. 4 (1 of 1)

PROJECT Preliminary Geotechnical Exploration, Proposed Codington County Justice Facility, US Highway 212, Watertown, SD

DEPTH in FEET	DESCRIPTION OF MATERIAL SURFACE ELEVATION <u>104.1 ft</u>	GEOLOGIC ORIGIN	N	WL	SAMPLE		LABORATORY TESTS							
					NO.	TYPE	WC	D	LL	PL	QU			
1 1/2	<b>LEAN CLAY:</b> very dark brown, frozen to 1' then moist, (CL)	TOPSOIL			1	HSA								
	<b>LEAN CLAY WITH SAND:</b> brown, moist, firm, (CL)	FINE ALLUVIUM	6		2	SPT	15	116					2200	
4 1/2	<b>SANDY LEAN CLAY:</b> a little gravel, mottled brown and gray, moist, firm, (CL)	MIXED ALLUVIUM	5		3	SPT								
6	<b>LEAN CLAY WITH SAND:</b> mottled brown and gray, moist, firm, (CL)	FINE ALLUVIUM			4	SPT	26	99						
9 1/2	<b>LEAN CLAY WITH SAND:</b> a little gravel, mottled brown and gray, moist, stiff to very stiff, (CL)	GLACIAL TILL	9		5	SPT								
			11		6	SPT								
			15		7	SPT								
			16		8	SPT								
21	Bottom of borehole at 21 feet.													

WATER LEVEL MEASUREMENTS

START 12-21-16 COMPLETE 12-21-16 1:54 pm

DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	WATER LEVEL	METHOD
12-21-16	2:33 pm	--	--	19	none	3.25" ID Hollow Stem Auger
--	--	--	--	--	--	
--	--	--	--	--	--	
--	--	--	--	--	--	CREW CHIEF Mike Wagner

GEOTECHNICAL TEST BORING 16-E56.GPJ GEOTEKENG.GDT 12/30/16







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**GEOTECHNICAL TEST BORING LOG**

GEOTEK # <u>16-E56</u>						BORING NO. <u>6 (1 of 1)</u>											
PROJECT <u>Preliminary Geotechnical Exploration, Proposed Codington County Justice Facility, US Highway 212, Watertown, SD</u>																	
DEPTH in FEET	DESCRIPTION OF MATERIAL SURFACE ELEVATION <u>101.1 ft</u>					GEOLOGIC ORIGIN	N	WL	SAMPLE		LABORATORY TESTS						
									NO.	TYPE	WC	D	LL	PL	QU		
	<b>LEAN CLAY:</b> very dark brown, frozen to 1' then moist, (CL)					TOPSOIL			1	HSA							
2½	<b>LEAN CLAY:</b> brown, moist, firm, (CL)					FINE ALLUVIUM	5		2	SPT	21	102					
4½	<b>LEAN CLAY WITH SAND:</b> a little gravel, mottled brown and gray, moist, firm to very stiff, (CL)					GLACIAL TILL	8		3	SPT	14	118					
							9		4	SPT							
							12		5	SPT							
							12		6	SPT							
							13		7	SPT							
							17		8	SPT							
21	Bottom of borehole at 21 feet.																
WATER LEVEL MEASUREMENTS						START <u>12-21-16</u> COMPLETE <u>12-21-16 10:37 am</u>											
DATE	TIME	SAMPLED DEPTH	CASING DEPTH	CAVE-IN DEPTH	WATER LEVEL	METHOD											
12-21-16	2:41 pm	21	--	17	none	3.25" ID Hollow Stem Auger											
--	--	--	--	--	--												
--	--	--	--	--	--												
--	--	--	--	--	--	CREW CHIEF Mike Wagner											

GEOTECHNICAL TEST BORING 16-E56.GPJ GEOTEKENG.GDT 12/30/16

# SOIL CLASSIFICATION CHART

MAJOR DIVISIONS			SYMBOLS		TYPICAL DESCRIPTIONS	
			GRAPH	LETTER		
<p><b>COARSE GRAINED SOILS</b></p> <p>MORE THAN 50% OF MATERIAL IS LARGER THAN NO. 200 SIEVE SIZE</p>	<p><b>GRAVEL AND GRAVELLY SOILS</b></p>	<p>CLEAN GRAVELS</p> <p>(LITTLE OR NO FINES)</p>		<b>GW</b>	WELL-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES	
		<p>GRAVELS WITH FINES</p> <p>(APPRECIABLE AMOUNT OF FINES)</p>		<b>GP</b>	POORLY-GRADED GRAVELS, GRAVEL - SAND MIXTURES, LITTLE OR NO FINES	
		<p>GRAVELS WITH FINES</p> <p>(APPRECIABLE AMOUNT OF FINES)</p>		<b>GM</b>	SILTY GRAVELS, GRAVEL - SAND - SILT MIXTURES	
		<p>GRAVELS WITH FINES</p> <p>(APPRECIABLE AMOUNT OF FINES)</p>		<b>GC</b>	CLAYEY GRAVELS, GRAVEL - SAND - CLAY MIXTURES	
	<p><b>SAND AND SANDY SOILS</b></p>	<p>CLEAN SANDS</p> <p>(LITTLE OR NO FINES)</p>		<b>SW</b>	WELL-GRADED SANDS, GRAVELLY SANDS, LITTLE OR NO FINES	
		<p>CLEAN SANDS</p> <p>(LITTLE OR NO FINES)</p>		<b>SP</b>	POORLY-GRADED SANDS, GRAVELLY SAND, LITTLE OR NO FINES	
		<p>SANDS WITH FINES</p> <p>(APPRECIABLE AMOUNT OF FINES)</p>		<b>SM</b>	SILTY SANDS, SAND - SILT MIXTURES	
		<p>SANDS WITH FINES</p> <p>(APPRECIABLE AMOUNT OF FINES)</p>		<b>SC</b>	CLAYEY SANDS, SAND - CLAY MIXTURES	
		<p><b>FINE GRAINED SOILS</b></p> <p>MORE THAN 50% OF MATERIAL IS SMALLER THAN NO. 200 SIEVE SIZE</p>	<p><b>SILTS AND CLAYS</b></p> <p>LIQUID LIMIT LESS THAN 50</p>		<b>ML</b>	INORGANIC SILTS AND VERY FINE SANDS, ROCK FLOUR, SILTY OR CLAYEY FINE SANDS OR CLAYEY SILTS WITH SLIGHT PLASTICITY
					<b>CL</b>	INORGANIC CLAYS OF LOW TO MEDIUM PLASTICITY, GRAVELLY CLAYS, SANDY CLAYS, SILTY CLAYS, LEAN CLAYS
	<b>OL</b>			ORGANIC SILTS AND ORGANIC SILTY CLAYS OF LOW PLASTICITY		
<p><b>SILTS AND CLAYS</b></p> <p>LIQUID LIMIT GREATER THAN 50</p>			<b>MH</b>	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SAND OR SILTY SOILS		
			<b>CH</b>	INORGANIC CLAYS OF HIGH PLASTICITY		
<p><b>HIGHLY ORGANIC SOILS</b></p>				<b>OH</b>	ORGANIC CLAYS OF MEDIUM TO HIGH PLASTICITY, ORGANIC SILTS	
<p><b>HIGHLY ORGANIC SOILS</b></p>				<b>PT</b>	PEAT, HUMUS, SWAMP SOILS WITH HIGH ORGANIC CONTENTS	

NOTE: DUAL SYMBOLS ARE USED TO INDICATE BORDERLINE SOIL CLASSIFICATIONS

# BORING LOG SYMBOLS AND DESCRIPTIVE TERMINOLOGY

## SYMBOLS FOR DRILLING AND SAMPLING

<u>Symbol</u>	<u>Definition</u>
Bag	Bag sample
CS	Continuous split-spoon sampling
DM	Drilling mud
FA	Flight auger; number indicates outside diameter in inches
HA	Hand auger; number indicates outside diameter in inches
HSA	Hollow stem auger; number indicates inside diameter in inches
LS	Liner sample; number indicates outside diameter of liner sample
N	Standard penetration resistance (N-value) in blows per foot
NMR	No water level measurement recorded, primarily due to presence of drilling fluid
NSR	No sample retrieved; classification is based on action of drilling equipment and/or material noted in drilling fluid or on sampling bit
SH	Shelby tube sample; 3-inch outside diameter
SPT	Standard penetration test (N-value) using standard split-spoon sampler
SS	Split-spoon sample; 2-inch outside diameter unless otherwise noted
WL	Water level directly measured in boring
▼	Water level symbol

## SYMBOLS FOR LABORATORY TESTS

<u>Symbol</u>	<u>Definition</u>
WC	Water content, percent of dry weight; ASTM:D2216
D	Dry density, pounds per cubic foot
LL	Liquid limit; ASTM:D4318
PL	Plastic limit; ASTM:D4318
QU	Unconfined compressive strength, pounds per square foot; ASTM:D2166

### DENSITY/CONSISTENCY TERMINOLOGY

<u>Density</u>	<u>Consistency</u>
<u>Term</u>	<u>Term</u>
<u>N-Value</u>	<u>Term</u>
Very Loose	Soft
Loose	Firm
Medium Dense	Stiff
Dense	Very Stiff
Very Dense	Hard

### PARTICLE SIZES

<u>Term</u>	<u>Particle Size</u>
Boulder	Over 12"
Cobble	3" – 12"
Gravel	#4 – 3"
Coarse Sand	#10 – #4
Medium Sand	#40 – #10
Fine Sand	#200 – #40
Silt and Clay	passes #200 sieve

### DESCRIPTIVE TERMINOLOGY

<u>Term</u>	<u>Definition</u>
Dry	Absence of moisture, powdery
Frozen	Frozen soil
Moist	Damp, below saturation
Waterbearing	Pervious soil below water
Wet	Saturated, above liquid limit
Lamination	Up to ½" thick stratum
Layer	½" to 6" thick stratum
Lens	½" to 6" discontinuous stratum

### GRAVEL PERCENTAGES

<u>Term</u>	<u>Range</u>
A trace of gravel	2-4%
A little gravel	5-15%
With gravel	16-50%